

# Drilled Pier Analysis (Bored Pile)

Organization: **CROWTHER REFERENCE**  
 Project Name: **P 126-134**  
 Job #:  
 Design by:  
 Date: **December 27, 2014**

## INPUT DATA

### Loading and Geometry

|                       |                      |                       |                            |
|-----------------------|----------------------|-----------------------|----------------------------|
| Units                 | English              | Pier Length           | 65.00 ft                   |
| Top of Pier Condition | Free Head            | Lateral Load (Shear)  | 12.00 kips                 |
| Passive Wedge         | 3.0                  | Moment Load           | 0.00 kip-ft                |
| Pier Diameter         | 1.00 ft              | Vertical Load         | 180.0 kips                 |
| Torsional Moment      | 0.00 kip-ft          | Uplift Load           | 90.0 kips                  |
| Conc. Strength (f'c)  | 4000 psi             | Groundwater Depth     | 3.00 ft                    |
| F.S. Skin Friction    | 2.0                  | F.S. Torsional Moment | 3.3                        |
| Shaft Type            | Vertical •<br>Belled | Cracked               | Uncracked •<br>50% cracked |

### Geotechnical Properties

| Lay # | Material Type | USCS | Layer Thick, ft | Consistency  | Lat. S.G, kcf   | Sk.Fr, psf | Kp   | F.S. on Kp | Cohesion, ksf | Gamma, pcf | Phi, Deg |
|-------|---------------|------|-----------------|--------------|-----------------|------------|------|------------|---------------|------------|----------|
| 1     | Granular Soil | SM   | 5.0<br>0 - 5    | Loose        | 115.0-<br>130.0 | 900        | 3.33 | 1.33       | 0.50          | 120.0      | 33.00    |
| 2     | Granular Soil | SM   | 12.0<br>5 - 17  | Medium Dense | 130.0-<br>275.0 | 1500       | 3.33 | 1.33       | 0.50          | 125.0      | 33.00    |
| 3     | Granular Soil | SM   | 5.0<br>17 - 22  | Loose        | 275.0-<br>130.0 | 900        | 3.33 | 1.33       | 0.50          | 120.0      | 29.00    |
| 4     | Cohesive Soil | CL   | 6.0<br>22 - 28  | Firm         | 130.0-<br>100.0 | 700        | 2.50 | 1.33       | 1.00          | 120.0      | 22.00    |
| 5     | Granular Soil | SM   | 15.0<br>28 - 43 | Medium Dense | 100.0-<br>230.0 | 1500       | 3.00 | 1.33       | 0.50          | 125.0      | 29.00    |
| 6     | Granular Soil | SC   | 5.0<br>43 - 48  | Loose        | 230.0-<br>140.0 | 900        | 3.00 | 1.33       | 0.75          | 120.0      | 25.00    |
| 7     | Granular Soil | SM   | 6.0<br>48 - 54  | Medium Dense | 140.0-<br>300.0 | 1500       | 3.30 | 1.33       | 0.50          | 125.0      | 32.00    |
| 8     | Granular Soil | SW   | 11.0<br>54 - 65 | Dense        | 300.0-<br>550.0 | 3000       | 4.00 | 1.33       | 0.50          | 130.0      | 36.00    |

### Results

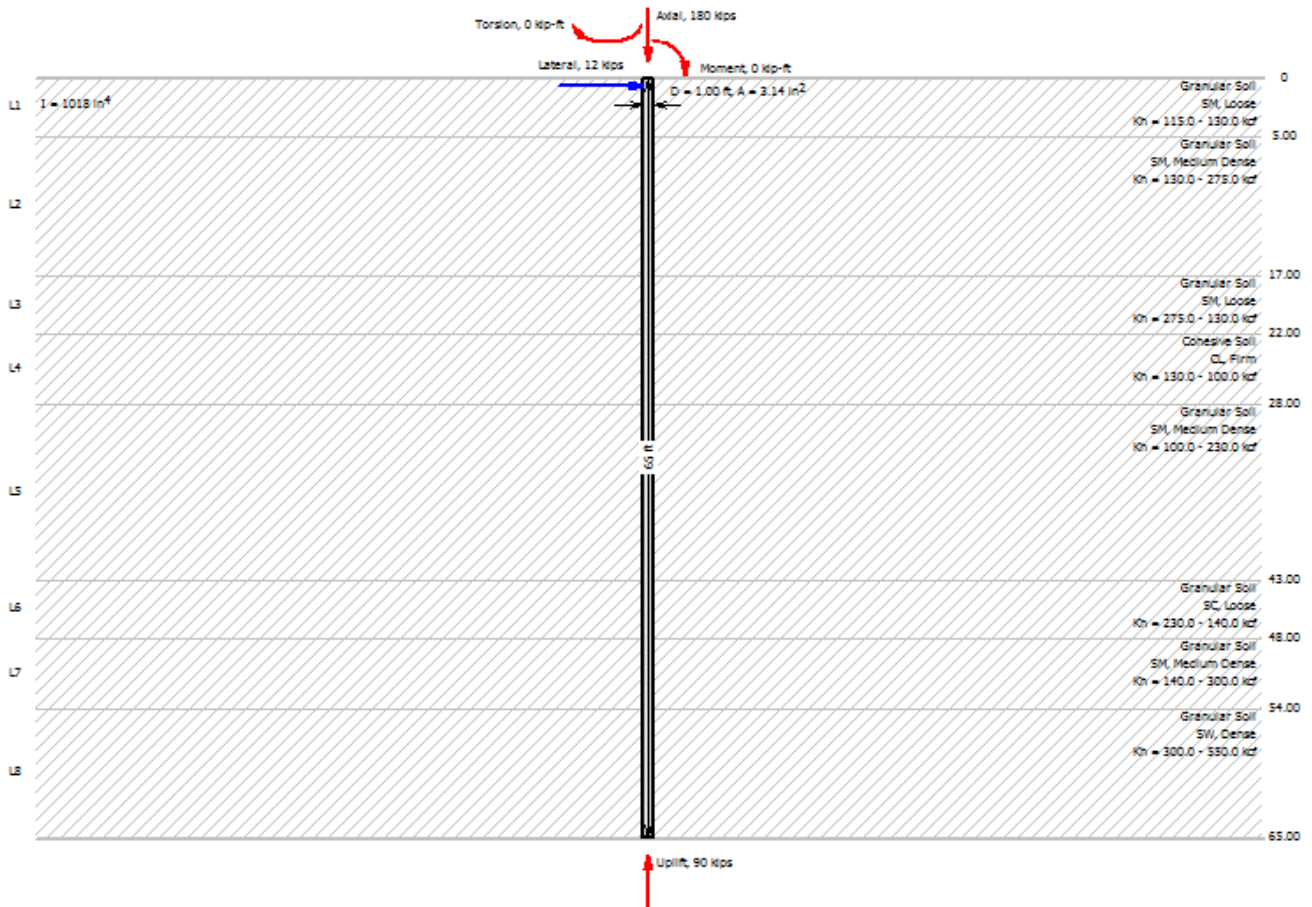
|                         |                       |                      |                       |
|-------------------------|-----------------------|----------------------|-----------------------|
| Conc. Elast. Modulus    | 3605 ksi              | Conc. Strength (f'c) | 4000 psi              |
| Cross Section Area      | 113.1 in <sup>2</sup> | Section Modulus      | 169.6 in <sup>3</sup> |
| Allowable Geot. Torsion | 165.4 k-ft            | Moment of Inertia    | 1018 in <sup>4</sup>  |

#### Allowable Structural Capacities

|                     |             |   |
|---------------------|-------------|---|
| Axial Compression   | 339.5 kips  | Maximum Deflection is 0.46 in at 0.00 ft              |
| Axial Tension       | -256.0 kips | Maximum Moment is 20.15 k-ft at 4.88 ft               |
| Torsional Moment    | 10.7 k-ft   | Maximum Shear is 12.00 kips at 0.00 ft                |
| Bending Moment (#1) | 39 k-ft     | Pier Tip Movement is 0.00 in at the bottom (65.00 ft) |

#### Allowable Geotechnical Capacities

|                     |            |                  |            |
|---------------------|------------|------------------|------------|
| Vertical Settlement | 0.32 in    | Axial Tension    | -90.2 kips |
| Axial Compression   | 180.3 kips | Torsional Moment | 165.4 k-ft |



**NOTES:**

1. Downdrag is a function of skin friction. If the user inputs a side friction of zero (designed for end bearing only), Downdrag will not be calculated by the program. Therefore, the user should independently calculate the Negative Skin Friction or Downdrag magnitude by other means.
2. The user shall ensure Positive side friction capacity + end bearing capacity (Geotechnical Axial Compression) exceeds axial load plus downdrag force (all service level). This may require additional pier embedment.
3. In reinforcement design,  $P_u$  shall equal to  $1.2(\text{Dead Load})$  plus  $1.6 (\text{Live Load} + \text{Downdrag Load})$ . These load factors may vary with time and governing code.

## Lateral Table

| Node # | Depth,<br>ft | Soil Modulus,<br>ksf | Shear,<br>kips | Moment,<br>kip-ft | Deflection,<br>in | Lat. Soil Pr,<br>k/ft^2 | Allow. Soil Pr,<br>k/ft^2 |
|--------|--------------|----------------------|----------------|-------------------|-------------------|-------------------------|---------------------------|
| 1      | 0.00         | 115                  | 12.00          | 0.00              | 0.46              | 4.40                    | 4.75                      |
| 2      | 1.63         | 120                  | 5.85           | 14.22             | 0.31              | 3.14                    | 6.21                      |
| 3      | 3.25         | 125                  | 1.63           | 19.95             | 0.19              | 2.01                    | 7.68                      |
| 4      | 4.88         | 130                  | -0.91          | 20.15             | 0.10              | 1.09                    | 9.14                      |
| 5      | 6.50         | 148                  | -2.22          | 16.50             | 0.03              | 0.39                    | 10.85                     |
| 6      | 8.13         | 168                  | -2.48          | 11.57             | 0.00              | -0.06                   | 12.38                     |
| 7      | 9.75         | 187                  | -2.13          | 6.94              | -0.02             | -0.29                   | 13.90                     |
| 8      | 11.38        | 207                  | -1.51          | 3.36              | -0.02             | -0.34                   | 15.43                     |
| 9      | 13.00        | 227                  | -0.90          | 1.03              | -0.02             | -0.29                   | 16.95                     |
| 10     | 14.63        | 246                  | -0.42          | -0.21             | -0.01             | -0.21                   | 18.48                     |
| 11     | 16.25        | 266                  | -0.11          | -0.68             | -0.01             | -0.12                   | 20.00                     |
| 12     | 17.88        | 250                  | 0.02           | -0.75             | 0.00              | -0.07                   | 20.86                     |
| 13     | 19.50        | 203                  | 0.05           | -0.77             | 0.00              | -0.05                   | 22.32                     |
| 14     | 21.13        | 155                  | 0.06           | -0.80             | 0.00              | -0.04                   | 23.79                     |
| 15     | 22.75        | 126                  | 0.08           | -0.81             | 0.00              | -0.03                   | 23.62                     |
| 16     | 24.38        | 118                  | 0.10           | -0.71             | 0.00              | -0.01                   | 24.72                     |
| 17     | 26.00        | 110                  | 0.11           | -0.59             | 0.00              | 0.00                    | 25.82                     |
| 18     | 27.63        | 102                  | 0.10           | -0.49             | 0.00              | 0.01                    | 26.92                     |
| 19     | 29.25        | 111                  | 0.08           | -0.27             | 0.00              | 0.01                    | 29.25                     |
| 20     | 30.88        | 125                  | 0.05           | -0.10             | 0.00              | 0.01                    | 30.62                     |
| 21     | 32.50        | 139                  | 0.02           | 0.00              | 0.00              | 0.01                    | 32.00                     |
| 22     | 34.13        | 153                  | 0.01           | 0.03              | 0.00              | 0.01                    | 33.37                     |
| 23     | 35.75        | 167                  | 0.00           | 0.04              | 0.00              | 0.00                    | 34.75                     |
| 24     | 37.38        | 181                  | 0.00           | 0.03              | 0.00              | 0.00                    | 36.12                     |
| 25     | 39.00        | 195                  | 0.00           | 0.02              | 0.00              | 0.00                    | 37.49                     |
| 26     | 40.63        | 209                  | 0.00           | 0.01              | 0.00              | 0.00                    | 38.87                     |
| 27     | 42.25        | 224                  | 0.00           | 0.00              | 0.00              | 0.00                    | 40.24                     |
| 28     | 43.88        | 214                  | 0.00           | 0.00              | 0.00              | 0.00                    | 42.39                     |
| 29     | 45.50        | 185                  | 0.00           | 0.00              | 0.00              | 0.00                    | 43.71                     |
| 30     | 47.13        | 156                  | 0.00           | 0.00              | 0.00              | 0.00                    | 45.03                     |
| 31     | 48.75        | 160                  | 0.00           | 0.00              | 0.00              | 0.00                    | 50.09                     |
| 32     | 50.38        | 203                  | 0.00           | 0.00              | 0.00              | 0.00                    | 51.60                     |
| 33     | 52.00        | 247                  | 0.00           | 0.00              | 0.00              | 0.00                    | 53.11                     |
| 34     | 53.63        | 290                  | 0.00           | 0.00              | 0.00              | 0.00                    | 54.62                     |
| 35     | 55.25        | 328                  | 0.00           | 0.00              | 0.00              | 0.00                    | 70.01                     |
| 36     | 56.88        | 365                  | 0.00           | 0.00              | 0.00              | 0.00                    | 71.91                     |
| 37     | 58.50        | 402                  | 0.00           | 0.00              | 0.00              | 0.00                    | 73.82                     |
| 38     | 60.13        | 439                  | 0.00           | 0.00              | 0.00              | 0.00                    | 75.73                     |
| 39     | 61.75        | 476                  | 0.00           | 0.00              | 0.00              | 0.00                    | 77.63                     |
| 40     | 63.38        | 513                  | 0.00           | 0.00              | 0.00              | 0.00                    | 79.54                     |
| 41     | 65.00        | 550                  | 0.00           | 0.00              | 0.00              | 0.00                    | 81.44                     |

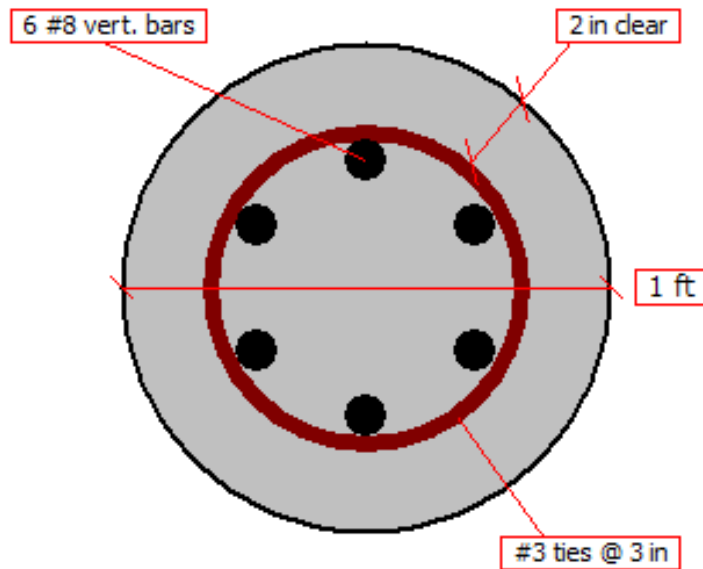
## Axial Table

| Layer # | Depth,<br>ft  | Unit wt,<br>pcf | Tot. Stress,<br>psf | Porewater,<br>psf | Eff. Stress,<br>psf | Skin Fric,<br>psf |
|---------|---------------|-----------------|---------------------|-------------------|---------------------|-------------------|
| 1       | 0.00 - 5.00   | 120.00          | 600.00              | 124.84            | 475.16              | 900               |
| 2       | 5.00 - 17.00  | 125.00          | 2100.00             | 873.88            | 1226.12             | 1500              |
| 3       | 17.00 - 22.00 | 120.00          | 2700.00             | 1185.98           | 1514.02             | 900               |
| 4       | 22.00 - 28.00 | 120.00          | 3420.00             | 1560.50           | 1859.50             | 700               |
| 5       | 28.00 - 43.00 | 125.00          | 5295.00             | 2496.80           | 2798.20             | 1500              |
| 6       | 43.00 - 48.00 | 120.00          | 5895.00             | 2808.90           | 3086.10             | 900               |
| 7       | 48.00 - 54.00 | 125.00          | 6645.00             | 3183.42           | 3461.58             | 1500              |
| 8       | 54.00 - 65.00 | 130.00          | 8075.00             | 3870.04           | 4204.96             | 3000              |

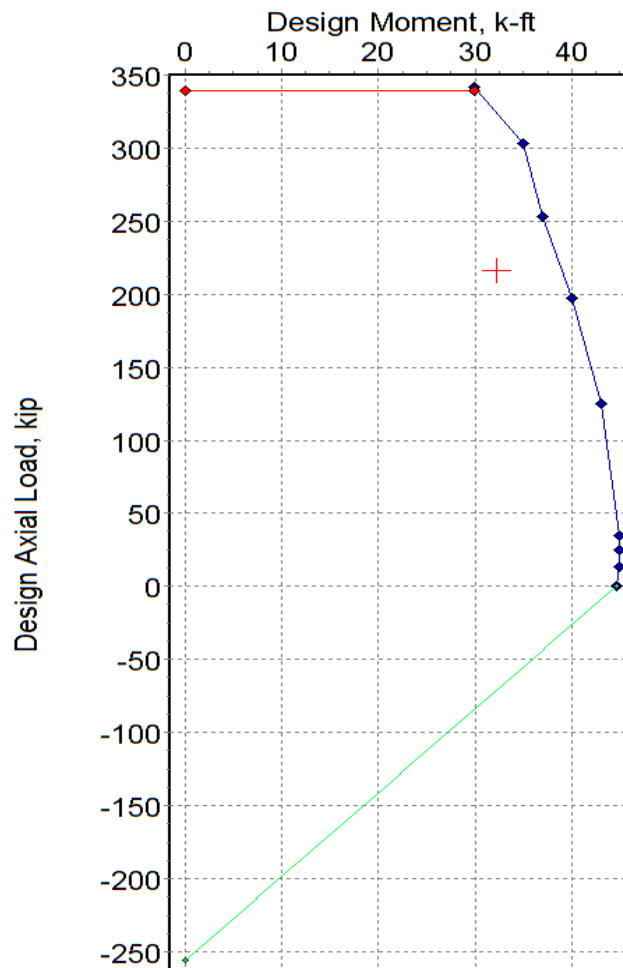
  

| Layer # | TSA Sk.Fr,<br>kips | Fr. Ang,<br>Deg | ESA Sk.Fr,<br>kips | Neg.Sk.Fr. | Downdrag,<br>kip | Beta | Geo. Tors. Cap,<br>kip-ft |
|---------|--------------------|-----------------|--------------------|------------|------------------|------|---------------------------|
| 1       | 9.9                | 33.00           | 1.5                | No         | 0.00             | 0.70 | 7.50                      |
| 2       | 56.5               | 33.00           | 12.0               | No         | 0.00             | 0.70 | 29.99                     |
| 3       | 14.1               | 29.00           | 9.3                | No         | 0.00             | 0.70 | 7.50                      |
| 4       | 13.2               | 22.00           | 18.5               | No         | 0.00             | 0.55 | 5.50                      |
| 5       | 70.7               | 29.00           | 30.4               | No         | 0.00             | 0.70 | 37.48                     |
| 6       | 14.1               | 25.00           | 20.4               | No         | 0.00             | 0.70 | 7.50                      |
| 7       | 28.3               | 32.00           | 38.2               | No         | 0.00             | 0.70 | 14.99                     |
| 8       | 84.8               | 36.00           | 89.1               | No         | 0.00             | 0.70 | 54.98                     |

## Reinforcement

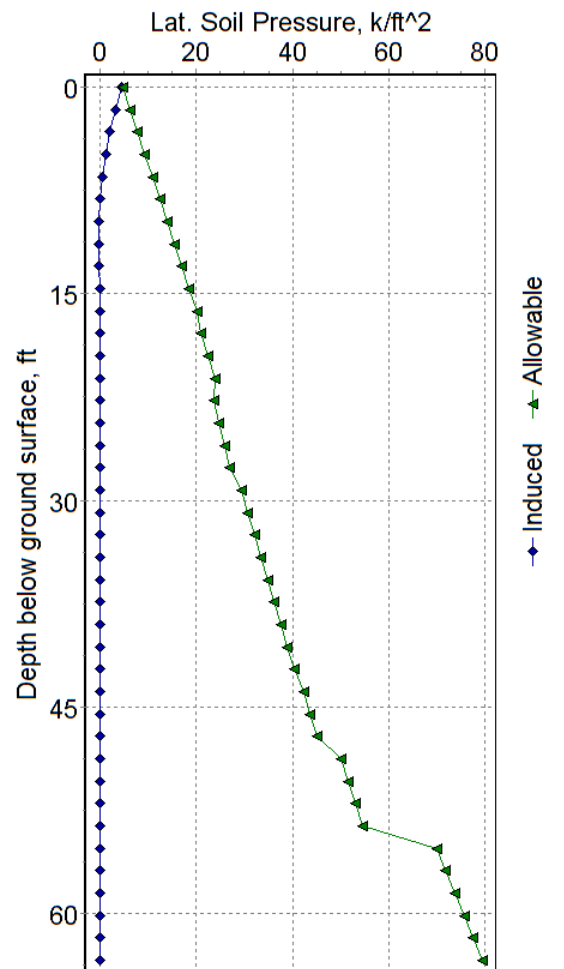
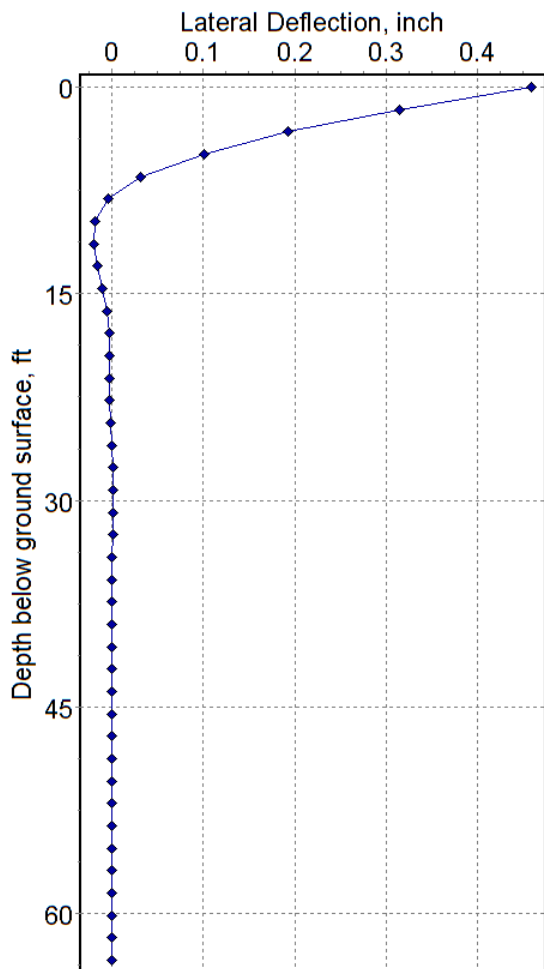
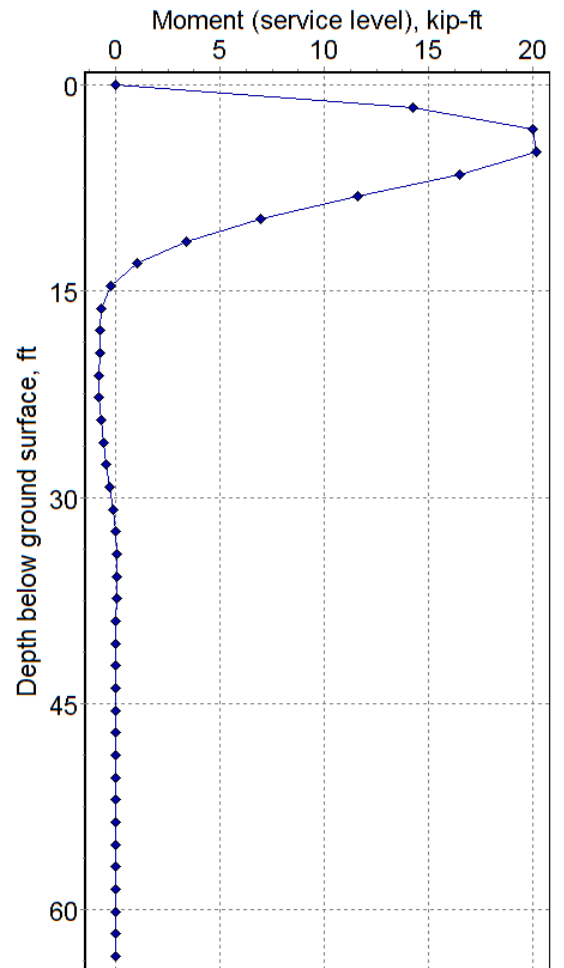
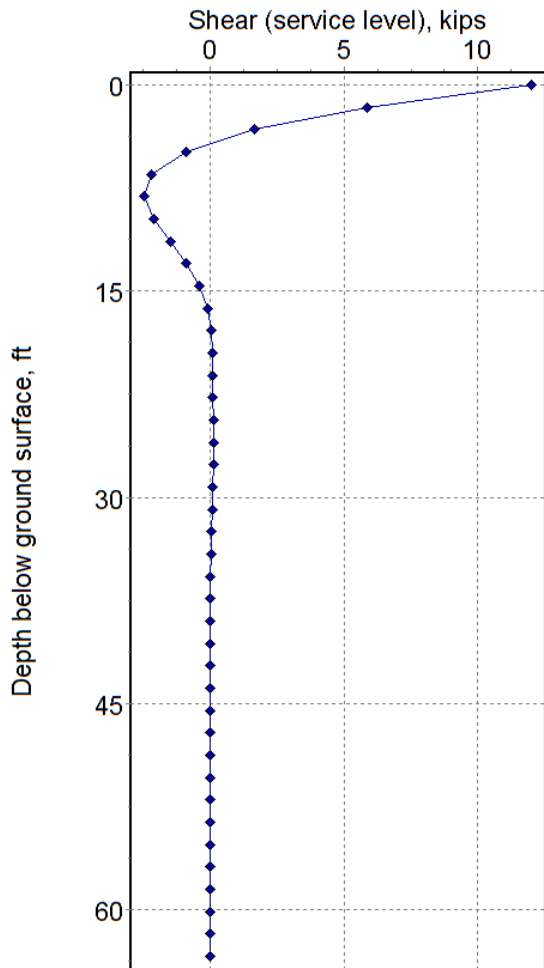


## Column Interaction Diagram



### NOTES:

1. If Torsional Moments are included, an additional check shall be made for concrete breakout due to shear directed parallel to the edge of anchors. Concrete Anchorage Breakout may govern.
2. If Torsional Moments are included in addition to Lateral Loads, Lateral capacity shall be reduced by 40% if Torsional Moment Load/Lateral Load ratio (TML/LL) is 15 and unaffected if TML/LL ratio is 7.5 or less. You may interpolate linearly for TML/LL ratio between 7.5 to 15. See Report # 4910-4504-723-12 by The University of Florida, April, 2003, available online.



## References:

1. "Analytical and Computer Methods in Foundation Engineering", J.E. Bowles, 1974
2. "Pile Foundation Analysis and Design", Poulos & Davis, 1980.
3. "Foundation Analysis", R.F. Scott, 1981
4. "Soil Mechanics in Engineering Practice", 2nd Ed, Terzaghi & Peck, 1967.
5. "Foundation Analysis and Design", 5th Ed., J.E. Bowles, 1996
6. "Foundation Design: Principles & Practices", 3rd Ed., Coduto, Kitch and Yeung, 2015
7. "Roark's Formulas for Stress and Strain", 7th Ed., W.C. Young & R.G. Budynas, 2002
8. "Engineering Design in Geotechnics" , 2nd Ed., F. Azizi, 2013
9. "Pile Design & Construction", Tomlinson & Woodward, 5th ed, 2007.
10. "Advanced Foundation Engineering" V.N.S. Murthy, 2007
11. "Analysis of Laterally Loaded Piles in Multilayered Soil Deposits", Basu, Salgado & Prezzi, 2008.
12. "Foundations and Earth Retaining Structures" M. Budhu, 2008
13. "Contemporary Topics in Deep Foundations", GSP 185, ASCE, 2009
14. "Single Piles and Pile Groups Under Lateral Loading", 2nd Ed., L.C. Reese & W.F. Van Impe, 2011
15. "Behaviour of Piles", Institute of Civil Engineers, 1971
16. "Load Testing of Deep Foundations", C. Crowther, 1988
17. "GeoSupport 2004", GSP 124, ASCE, 2004
18. "Design and Construction of Drilled Piers", American Concrete Institute 336.3R, 2006
19. "Drilled Shafts: Construction Procedures and LRFD Design Methods", FHWA-NHI, 2010
20. "Drilled Shafts-Student Workbook", L.C. Reese & M.W. O'Neil, NHI Course 13214, 1988
21. "Handbook of Soil Mechanics", A. Kezdi & L. Rethati, Vol. 3 & 4, 1990
22. "Building Code Requirements for Structural Concrete", ACI 318-11
23. "Reinforced Concrete Structures- Analysis & Design", D.A. Fanella, 2011
24. "Determine Optimum Depths of Drilled Shafts Subject to Combined Torsion and Lateral Loads Using Centrifuge Testing", University of Florida Report # 4910-4504-723-12, April 2003.
25. "Geotechnical Engineering: Unsaturated & Saturated Soils", Jean-Louis Briaud, 2013
26. "Pile Downdrag During Construction of Two Bridge Abutments", Brian K. Sears, 2008
27. Drilled Pier v1.0 Software by SoilStructure.com, 2015